# Detailed Solutions <br> GATE 2020 

## Civil Engineering <br> Forenoon Session - 09.02.2020

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## GATE PAPER ANALYSIS-2020, $9^{\text {th }}$ Feb. Forenoon

## Givil Engineering

| Subject | Number of <br> Questions |  | Level of Difficulty |
| :---: | :---: | :---: | :---: |
|  | 1 M | 2 M |  |
| Geo-technical Engineering | 1 | 7 | Moderate |
| RCC Design | 1 | 2 | Easy |
| Structural Analysis | 2 | 3 | Easy |
| Strength of Materials | 2 | 1 | Easy |
| Fluid Mechanics | 5 | 2 | Moderate |
| Environmental engineering | 2 | 5 | Moderate |
| Hydrology \& Irrigation | 2 | 1 | Moderate |
| Transportation Engineering | 4 | 2 | Difficult |
| Surveying | 0 | 2 | Moderate |
| Steel | 0 | 1 | Moderate |
| Engineering Mechanics | 2 | 0 | Easy |
| CPM / PERT | 0 | 0 |  |
| Engineering Mathematics | 4 | 4 | Moderate |
| General Aptitude | 5 | 5 | Easy |

## General Aptitude

## Question 1

Select the word that fits the analogy :
Fuse : Fusion :: Use : $\qquad$
(A) Usage
(B) User
(C) Uses
(D) Usion

Ans. (A)

## Question 2

Insert seven numbers between 2 and 34, such that the resulting sequence including 2 and 34 is an arithmetic progression. The sum of these inserted seven numbers is $\qquad$ .
(A) 130
(B) 120
(C) 124
(D) 126

Ans. (D)
Sol. The resulting sequence after inserting 7 terms

$$
\begin{aligned}
& 2, A_{1}, A_{2}, A_{3}, \ldots \ldots, A_{7}, 34 \\
& T_{9}=34 \\
& a+(n-1) d=34 \\
& 2+(9-1) d=34 \\
& 8 d=32 \\
& d=4
\end{aligned}
$$

So, the inserted sequence

$$
\begin{aligned}
& 6,10,14,18,22,26,30 \\
& \operatorname{Sum}=\left(\frac{6+30}{2}\right) \times 7=126
\end{aligned}
$$

Hence, the correct option is (D).

## Question 3

The sum of two positive numbers is 100 . After subtracting 5 from each number, the product of the resulting numbers is 0 . One of the original numbers is $\qquad$
(A) 90
(B) 80
(C) 95
(D) 85

Ans. (C)
Sol. Let, two positive numbers are a and b ,
According to question,

$$
\begin{align*}
& a+b=100  \tag{i}\\
& (a-5)(b-5)=0 \tag{ii}
\end{align*}
$$

$(a-5)(b-5)$ is zero only when $(a-5)$ or $(b-5)$ is zero

Assuming,

$$
\begin{aligned}
& (a-5)=0 \\
& a=5 \\
& b=95
\end{aligned}
$$

Hence, the correct option is (C).

## Question 4

If $0,1,2, \ldots, 7,8,9$ are coded as $\mathrm{O}, \mathrm{P}, \mathrm{Q}, \ldots, \mathrm{V}, \mathrm{W}, \mathrm{X}$, then 45 will be coded as $\qquad$ .
(A) SU
(B) ST
(C) SS
(D) TS

Ans. (B)
Sol. The given codes are

|  |
| :---: |
| 2 |
|  |
|  |
|  |
|  |
|  |
|  |
|  |

So, the codes for $45 \rightarrow S T$
Hence, the correct option is (B).

## Question 5

The total expenditure of a family, on different activities in a month, is shown in the pie-chart. The extra money spent on education as compared to transport (in percent) is $\qquad$ .

(A) 100
(B) 50
(C) 33.3
(D) 5

Ans. (B)
Sol. $\quad$ Money spent on education $=15 \%$
Money spent on transport $=10 \%$
$\frac{\text { Money spent on eduction }}{\text { Money spent on transport }}=\frac{15}{10}=1.5$

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Money spent on education $=1.5$ (Money spent on transport)
So, money spent on education is $50 \%$ more than money spent on transport.
Hence, the correct option is (B).

## Question 6

Five friends $\mathrm{P}, \mathrm{Q}, \mathrm{R}, \mathrm{S}$ and T went camping. At night, they had to sleep in a row inside the tent. $\mathrm{P}, \mathrm{Q}$, and T refused to sleep next to $R$ since he snored loudly. $P$ and $S$ wanted to avoid $Q$ as he usually hugged people in sleep.
Assuming everyone was satisfied with the sleeping arrangements, what is the order in which they slept?
(A) RSPTQ
(B) QRSPT
(C) QTSPR
(D) SPRTQ

Ans. (A)
Sol. Option (B) RSPTQ is correct choice because both conditions are fulfilled in option (B)

1. $P, Q$ and $T$ are not next to $R$.
2. $P$ and $S$ are not next to $Q$.

Hence, the correct option is (A).

## Question 7

The unit's place in $26591749^{110016}$ is $\qquad$ .
(A) 9
(B) 3
(C) 1
(D) 6

Ans. (C)
Sol.

$$
\begin{aligned}
(26591749)^{110016} & =(26591749)^{2 k} \\
& =9^{2} \\
& =81
\end{aligned}
$$

So, the unit digit is 1
Hence, the correct option is (C).

## Question 8

His hunger for reading is insatiable. He reads indiscriminately. He is most certainly a/an $\qquad$ reader.
(A) voracious
(B) precocious
(C) wise
(D) all-round

Ans. (A)
Sol. Voracious : Engaging in an activity with great eagerness or enthusiasm.

## Question 9

The American psychologist Howard Gardner expounds that human intelligence can be sub-categorised into multiple kinds, in such a way that individuals differ with respect to their relative competence in each kind. Based on this theory, modern educationists insist on prescribing multi-dimensional curriculum and evaluation parameters that enable development and assessment of multiple intelligences.
Which of the following statements can be inferred from the given text?
(A) Howard Gardner insists that the teaching curriculum and evaluation needs to be multi-dimensional.

(B) Modern educationists insist that the teaching curriculum and evaluation needs to be multidimensional.
(C) Modern educationists want to develop and assess the theory of multiple intelligences.
(D) Howard Gardner wants to develop and assess the theory of multiple intelligences.

Ans. (A)
Question 10
It is a common criticism that most of the academicians live in their $\qquad$ , so, they are not aware of the real life challenges.
(A) glass palaces
(B) homes
(C) big flats
(D) ivory towers

Ans. (D)
Sol. To live or be in ivory tower means "that not to known about or to want to avoid the orginary and unpleasant things that happen in people's life".

## Technical Section

## Question 1

The value of $\lim _{x \rightarrow \infty} \frac{x^{2}-5 x+4}{4 x^{2}+2 x}$ is
(A) $\frac{1}{4}$
(B) 1
(C) $\frac{1}{2}$
(D) 0

Ans. (A)
Sol. Applying L-hospital rule

$$
\begin{aligned}
& =\lim _{x \rightarrow \infty}\left(\frac{2 x-5}{8 x+5}\right)=\frac{\infty}{\infty} \\
& =\lim _{x \rightarrow \infty}\left(\frac{2}{8}\right)=\frac{1}{4}
\end{aligned}
$$

Question 2
An amount of 35.67 mg HCl is added to distilled water and the total solution volume is made to one litre. The atomic weights of H and Cl are 1 and 35.5 , respectively. Neglecting the dissociation of water, the pH of the solution, is
(A) 2.50
(B) 3.50
(C) 2.01
(D) 3.01

Ans. (D)
Sol.
$\mathrm{HCl} \rightarrow \mathrm{H}^{+}+\mathrm{Cl}^{-}$

$$
\begin{aligned}
& 36.5 \rightarrow 1 \\
& 1 \text { unit } \rightarrow \frac{1}{36.5}
\end{aligned}
$$

Given 35.67 mg of HCl s

$$
\begin{aligned}
& H^{+} \rightarrow \frac{1}{36.5} \times 35.67 \\
& H^{+} \rightarrow 0.9772 \mathrm{mg}
\end{aligned}
$$

For $\mathrm{pH}\left[\mathrm{H}^{+}\right]$should be in $\mathrm{mol} /$ litre

$$
\begin{aligned}
& {\left[H^{+}\right]=\frac{0.9772}{1} \times 10^{-3}} \\
& p H=-\log _{10}\left[H^{+}\right] \\
& p H=-\log _{10}\left[0.9772 \times 10^{-3}\right] \\
& p H=3.01
\end{aligned}
$$

## Question 3

Consider the planar truss shown in the figure (not drawn to the scale)


Neglecting self-weight of the members, the number of zero-force members in the truss under the action of the load $P$, is
(A) 7
(B) 6
(C) 8
(D) 9

Ans. (C)
Sol.


Take joint A


$$
\begin{aligned}
& \Sigma F_{x}=0, \quad \Sigma F_{y}=0 \\
& F_{A C}=0, F_{A B}=0
\end{aligned}
$$

Similarly, take joint $B$

$$
F_{B D}=F_{B C}=0
$$

Similarly, take joint C

$$
F_{C E}=F_{C D}=0
$$

Take joint D


$$
\begin{aligned}
& \Sigma F_{y}=0, \Sigma F_{y}=0 \\
& F_{D E}=P, F_{D F}=0
\end{aligned}
$$

Truss reduce (it is 1 degree indeterminate)


Take redundant as vertical support


Applied unit load at 1 as shown in figure (at redundant)


$$
\begin{aligned}
& X=-\frac{\Sigma P U l}{\Sigma U^{2} l}=-\frac{P \times 1 \times l}{1^{2} \times l} \\
& X=-P
\end{aligned}
$$

Force in member $G F=F_{G F}+U x$
Force in member $G F=F_{G F}+U x$

$$
\begin{aligned}
& =P-(-P \times 1) \\
& =0
\end{aligned}
$$



No. of zero force member is 8 .

## Question 4

A road in a hilly terrain is to be laid at a gradient of $4.5 \%$. A horizontal curve of radius 100 m is laid at a location on this road. Gradient needs to be eased due to combination of curved horizontal and vertical profiles of the road. As per IRC, the compensated gradient (in $\%$, round off to one decimal place), is
$\qquad$ -.

Ans. (4)
Sol. $\quad$ Gradient $=4.5 \%$
Radius $(R)=100 \mathrm{~m}$
Grade compensation $\left.=\frac{30+R}{R}\right\}$

$$
\begin{aligned}
& \ngtr \frac{75}{R} \\
& =\frac{30+100}{100} \ngtr \frac{75}{100}=1.3 \% \ngtr 0.75
\end{aligned}
$$

G.C. $=0.75$

Compensated Gradient $=$ Gradient $-\mathrm{G} . \mathrm{C}$.

$$
\begin{aligned}
& =4.5 \%-0.75 \\
& =3.75 \% \nless 4 \%
\end{aligned}
$$

Hence Compensated gradient $=4 \%$

## Question 5

A planar elastic structure is subjected to uniformly distributed load, as shown in the figure (not drawn to the scale)



Neglecting self-weight, the maximum bending moment generated in the structure (in kN.m, round off to the nearest integer), is $\qquad$ .
Ans. 96
Sol. $V_{A}=V_{B}=\frac{w L}{2}=\frac{12 \times 8}{2}=48 \mathrm{kN}$
As horizontal thrust is zero so it behaves like a beam (curved beam)

$$
M_{\max }=\frac{w L^{2}}{8}(\mathrm{At} \text { crown })=\frac{12 \times 8^{2}}{8}=96 \mathrm{kNm}
$$

## Question 6

The probability that a 50 year flood may NOT occur at all during 25 years life of a project (round off to two decimal places), is $\qquad$ -
Ans. (0.603)
Sol. $\quad T=50$ year

$$
\begin{aligned}
& P=\frac{1}{50} \\
& q=\left(1-\frac{1}{50}\right)
\end{aligned}
$$

For food may not occur

$$
\begin{aligned}
& P={ }^{n} C_{r} P^{r} q^{n-r} \\
& r=0 \text { and } n=25 \\
& P={ }^{25} C_{0}\left(\frac{1}{50}\right)^{0}\left(1-\frac{1}{50}\right)^{25} \\
& P=\left(\frac{49}{50}\right)^{25} \\
& P=0.603
\end{aligned}
$$

## Question 7

Velocity of flow is proportional to the first power of hydraulic gradient in Darcy's law. This law is applicable to
(A) transitional flow in porous media
$\frac{\text { P1 }-1+1|+1|}{\text { steps to success... }}$
(B) laminar flow in porous media
(C) laminar as well as turbulent flow in porous media
(D) turbulent flow in porous media

Ans. (B)
Sol. Darcy's law is applicable only for laminar flow in porous medium.
Hence, the correct option is (B).

## Question 8

During chlorination process, aqueous (aq) chlorine reacts rapidly with water to form $\mathrm{Cl}^{-}, \mathrm{HOCl}$, and $\mathrm{H}^{+}$ as shown below

$$
\mathrm{Cl}_{2}(\mathrm{aq})+\mathrm{H}_{2} \mathrm{O} \rightleftharpoons \mathrm{HOCl}+\mathrm{Cl}^{-}+\mathrm{H}^{+}
$$

The most active disinfectant in the chlorination process from amongst the following, is
(A) $\mathrm{H}^{+}$
(B) $\mathrm{H}_{2} \mathrm{O}$
(C) $\mathrm{Cl}^{-}$
(D) HOCl

Ans. (D)
Sol.

$$
\mathrm{Cl}_{2}+\mathrm{H}_{2} \mathrm{O} \underset{\substack{\text { hypochlorous } \\ \text { acid }}}{\stackrel{\text { pH }}{ } \mathrm{HO}}+\mathrm{HCl}
$$

$\mathrm{HOCl}=\stackrel{\mathrm{pH}>8}{\rightleftharpoons} \underset{\mathrm{pH}<7}{\rightleftharpoons} \mathrm{H}^{+}+\mathrm{OCl}^{-}$
(Unstable, hence
breakdown at higher
pH )

- At $\mathrm{pH}<5$, chlorine does not react with water and remains as free chlorine.
- ( $\mathrm{HOCl}+\mathrm{OCl}^{-}+\mathrm{Cl}_{2}$ ) are combinedly called freely available chlorine. Out of these forms of freely available chlorine, HOCl is most destructive. It is $80 \%$ more effective than $\mathrm{OCl}^{-}$ion. Hence, pH of water should be maintained slightly below 7 .


## Question 9

In a two-dimensional stress analysis, the state of stress at a point $P$ is

$$
[\sigma]=\left[\begin{array}{ll}
\sigma_{x x} & \tau_{x y} \\
\tau_{x y} & \sigma_{y y}
\end{array}\right]
$$

The necessary and sufficient condition for existence of the state of pure shear at the point $P$, is
(A) $\sigma_{x x}+\sigma_{y y}=0$
(B) $\tau_{x y}=0$
(C) $\sigma_{x x} \sigma_{y y}-\tau_{x y}^{2}=0$
(D) $\left(\sigma_{x x}-\sigma_{y y}\right)^{2}+4 \tau_{x y}^{2}=0$

Ans. (A)
Sol. Given,

$$
\sigma=\left[\begin{array}{ll}
\sigma_{x x} & \tau_{x y} \\
\tau_{x y} & \sigma_{y y}
\end{array}\right]
$$

For the state of pure shear, the centre of Mohr's circle coincides with origin. Also the principal stresses are equal to shear stress
Hence,

$$
\frac{\sigma_{x x}+\sigma_{y y}}{2} \pm \sqrt{\left(\frac{\sigma_{x x}-\sigma_{y y}}{2}\right)^{2}+\tau_{x y}^{2}}=\sqrt{\left(\frac{\sigma_{x x}-\sigma_{y y}}{2}\right)^{2}+\tau_{x y}^{2}}
$$

Let us take positive sign into consideration,

$$
\begin{aligned}
& \frac{\sigma_{x x}+\sigma_{y y}}{2}=0 \\
& \sigma_{x x}+\sigma_{y y}=0
\end{aligned}
$$

## Question 10

During the process of hydration of cement, due to increase in Dicalcium Silicate $\left(\mathrm{C}_{2} \mathrm{~S}\right)$ content in cement clinker, the heat of hydration
(A) increases
(B) initially decreases and then increases
(C) does not change
(D) decreases

Ans. (D)
Sol. Due to increase in $C_{2} S$ heat of hydration decreases.

## Question 11

In an urban area, a median is provided to separate the opposing streams of traffic. As per IRC : 86-1983, the desirable minimum width (in m , expressed as integer) of the median, is $\qquad$ .
Ans. 5
Sol. As per IRC : 86-1983
Desirable minimum width of median in urban roads $=5 \mathrm{~m}$

## Question 12

The data for an agricultural field for a specific month are given below :

| Pan Evaporation | $=100 \mathrm{~mm}$ |
| :--- | :--- |
| Effective Rainfall | $=20 \mathrm{~mm}$ (after deducting losses due to runoff and deep percolation) |
| Crop Coefficient | $=0.4$ |
| Irrigation Efficiency | $=0.5$ |

The amount of irrigation water (in mm ) to be applied to the field in that month, is
(A) 40
(B) 0
(C) 80
(D) 20

Ans. (A)
Sol. Water required by crop $=100 \times 0.4 \mathrm{~mm}=40 \mathrm{~mm}$
Effective rainfall $=20 \mathrm{~mm}$
Additional water required $=20 \mathrm{~mm}$
Amount of water required after accounting irrigation efficiency $=\frac{20}{0.5}=40 \mathrm{~mm}$

## Question 13

A reinforcing steel bar, partially embedded in concrete, is subjected to a tensile force $P$. The figure that appropriately represents the distribution of the magnitude of bond stress (represented as hatched region), along the embedded length of the bar, is
(A)

(B)

(C)

(D)


Ans. (D)
Q. 14 Uniform flow with velocity $U$ makes an angle $\theta$ with the $y$-axis, as shown in the figure


The velocity potential $(\phi)$, is
(A) $\pm U(x \sin \theta+y \cos \theta)$
(B) $\pm U(x \sin \theta-y \cos \theta)$
(C) $\pm U(y \sin \theta-x \cos \theta)$
(D) $\pm U(y \sin \theta+x \cos \theta)$

Ans. (A)
Sol. Velocity in $x$-depth, $u_{x}=u \sin \theta$
Velocity in $y$-depth, $u_{y}=u \cos \theta$

$$
-\frac{\partial \phi}{\partial x}=u_{x}
$$

Integrating it,

$$
\begin{align*}
& \phi=-u_{x} x+f(x)+C \\
& =-(u \sin \theta) x+f(y)+C  \tag{i}\\
& =\frac{-\partial \phi}{\partial y}=u_{y}
\end{align*}
$$

By equation (i) and (ii),

$$
\phi=-u(x \sin \theta+y \cos \theta)
$$

If we take $\frac{\partial \phi}{\partial x}=u_{x}$ and $\frac{\partial \phi}{\partial y}=u_{y}$
Then, $\phi=u(x \sin \theta+y \cos \theta)$

$$
\phi= \pm u(x \sin \theta+y \cos \theta)
$$

Q. 15 A 4 m wide rectangular channel carries $6 \mathrm{~m}^{3} / \mathrm{s}$ of water. The Manning's ' n ' of the open channel is 0.02 . Considering $g=9.81 \mathrm{~m} / \mathrm{s}^{2}$, the critical velocity of flow (in $\mathrm{m} / \mathrm{s}$, round off to two decimal places) in the channel, is $\qquad$ .
Ans. (2.45)
Sol.

$$
Q=6 \mathrm{~m}^{3} / \mathrm{s}, B=4 \mathrm{~m}, n=0.02, g=9.81 \mathrm{~m} / \mathrm{s}^{2}
$$

For critical flow, $f_{r}=1$

$$
\begin{aligned}
& f_{r}=\frac{V_{c}}{\sqrt{g_{y c}}}=1 \\
& y_{c}=\left(\frac{q^{2}}{g}\right)^{1 / 3} \\
& q=\frac{Q}{B}=\frac{6}{4}=1.5 \mathrm{~m}^{3}
\end{aligned}
$$

$$
\begin{aligned}
& y_{c}=\left(\frac{(1.5)^{2}}{9.81}\right)^{1 / 3}=0.612 \mathrm{~m} \\
& V_{c}=\sqrt{g_{y c}}=\sqrt{9.81 \times 0.612}=2.4 \mathrm{~m} / \mathrm{s}
\end{aligned}
$$

Q. 16 In the following partial differential equation, $\theta$ is a function of $t$ and $z$, and $D$ and $K$ are functions of $\theta$
$D(\theta) \frac{\partial^{2} \theta}{\partial z^{2}}+\frac{\partial K(\theta)}{\partial z}-\frac{\partial \theta}{\partial t}=0$
The above equation is
(A) a second order non-linear equation
(B) a second degree linear equation
(C) a second degree non-linear equation
(D) a second order linear equation

Ans. (A)
Sol. In first term of given $D=f(\theta)$
Equation contains product of dependent variable with it's derivative, so it is non-linear.
We have $2^{\text {nd }}$ order derivative hence, option (A) second degree non-linear equation.
Q. 17 A river has a flow of 1000 million litres per day (MLD), $\mathrm{BOD}_{5}$ of $5 \mathrm{mg} / \mathrm{litre}$ and Dissolved Oxygen (DO) level of $8 \mathrm{mg} /$ litre before receiving the wastewater discharge at a location. For the existing environmental conditions, the saturation DO level is $10 \mathrm{mg} / \mathrm{litre}$ in the river. Wastewater discharge of 100 MLD with the $\mathrm{BOD}_{5}$ of $200 \mathrm{mg} / \mathrm{litre}$ and DO level of $2 \mathrm{mg} /$ litre falls at that location. Assuming complete mixing of wastewater and river water, the immediate DO deficit (in $\mathrm{mg} / \mathrm{litre}$, round off to two decimal places), is
$\qquad$ .
Ans. (2.55)
Sol. $\quad D O_{\text {mix }}=\frac{D O_{s} Q_{s}+D O_{R} Q_{R}}{Q_{s}+Q_{R}}$

$$
=\frac{2 \times 100+8 \times 1000}{100+1000}=7.45 \mathrm{mg} / 1
$$

$D O=D O_{\text {sat }}-D O_{\text {mix }}$
$=10-7.45$
$=2.545 \mathrm{mg} / \mathrm{l}$
Q. 18 A body floating in a liquid is in a stable state of equilibrium if its
(A) Metacentre lies above its centre of gravity
(B) Metacentre lies below its centre of gravity
(C) Metacentre coincides with its centre of gravity
(D) Centre of gravity is below its centre of buoyancy

Ans. (A)
Q. 19 In a drained triaxial compression test, a sample of sand fails at deviator stress of 150 kPa under confining pressure of 50 kPa . The angle of internal friction (in degree, round off to the nearest integer) of the sample, is $\qquad$ .
Ans. (37)

## Sol. Given :

$$
\begin{aligned}
& \sigma_{3}=50 \mathrm{kPa} \\
& \sigma_{1}=\sigma_{3}+\sigma_{d} \\
& =150+50 \\
& =200 \mathrm{kPa}
\end{aligned}
$$



$$
\begin{aligned}
& \sigma_{1}=\sigma_{3} \frac{1+\sin \phi}{1-\sin \phi}+2 C \sqrt{\frac{1+\sin \phi}{1-\sin \phi}}^{0} \quad\{b c z . c=0\} \\
& 200=50 \frac{1+\sin \phi}{1-\sin \phi} \\
& 4=\tan ^{2}\left(45^{\circ}+\frac{\phi}{2}\right) \\
& 2=\tan \left(45^{\circ}+\frac{\phi}{2}\right) \\
& \tan ^{-1} 2=\left(45+\frac{\phi}{2}\right) \\
& \therefore \quad \phi=36.87^{\circ}
\end{aligned}
$$

So, the angle of internal friction to the nearest integer is $37^{\circ}$.
Q. 20 The area of an ellipse represented by an equation $\frac{x^{2}}{a^{2}}+\frac{y^{2}}{b^{2}}=1$ is
(A) $\pi a b$
(B) $\frac{\pi a b}{4}$
(C) $\frac{4 \pi a b}{3}$
(D) $\frac{\pi a b}{2}$

Ans. (A)
Sol.
$\frac{\text { H1 }-1+1 \mid+1}{\text { steps to success... }}$


$$
\begin{aligned}
& \text { Area }=\iint(1) d y d x=\int_{x=-a}^{a} \int_{y=-\frac{b}{a} \sqrt{-x^{2}+a^{2}}}^{\frac{b-\sqrt{-x^{2}+a^{2}}}{}}(1) d y d x \\
&=4 \int_{x=0}^{a} \int_{y=0}^{\frac{b}{a} \sqrt{a^{2}-x^{2}}}(1) d y d x \\
&=4 \int_{x=0}^{a} \int_{y=0}^{\frac{b}{a} \sqrt{a^{2}-x^{2}}} d x \\
&=\pi a b
\end{aligned}
$$

Q. 21 The true value of $\ln (2)$ is 0.69 . If the value of $\ln (2)$ is obtained by linear interpolation between $\ln (1)$ and $\ln (6)$, the percentage of absolute error (round off to the nearest integer), is
(A) 35
(B) 69
(C) 48
(D) 84

Ans. (C)
Sol. True value $\ln (2)=0.69$
$\ln (1)$
$\ln (6)$


Method of interpolation
Actual value $=\frac{1.79-0}{(6-1)} \times(2-1)=0.358$
$\%$ error $=\frac{\text { True value }- \text { Actual value }}{\text { True value }} \times 100$

$$
\begin{aligned}
& =\frac{0.69-0.358}{0.69} \times 100 \\
& =48.11 \%
\end{aligned}
$$

Q. 22 In a soil investigation work at a site, Standard Penetration Test (SPT) was conducted at every 1.5 m interval up to 30 m depth. At 3 m depth, the observed number of hammer blows for three successive 150 mm penetrations were 8,6 and 9 , respectively. The SPT N- value at 3 m depth, is
(A) 17
(B) 15
(C) 14
(D) 23

Ans. (B)
Sol. No, of blows for each 150 mm penetration 8, 6 and 9
Not consider first 150 mm number of blows.
Hence, for last 300 mm , number of blows are 15.
Hence, observed SPT number $=15$
Q. 23 The Los Angles test for stone aggregates is used to examine
(A) soundness
(B) abrasion resistance
(C) specific gravity
(D) crushing strength

Ans. (B)
Q. 24 Which one of the following statements is NOT correct?
(A) The ultimate bearing capacity of a strip foundation supported on the surface of sandy soil increases in direct proportion to the width of footing.
(B) A clay deposit with a liquidity index greater than unity is in a state of plastic consistency.
(C) The cohesion of normally consolidated clay is zero when triaxial test is conducted under consolidated undrained condition.
(D) In case of a point load, Boussinesq's equation predicts higher value of vertical stress at a point directly beneath the load as compared to Westergaard's equation.
Ans. (B)
Sol. A clay deposit with liquidty index greater than 1, will be in liquid stage of consistency.

$$
\begin{aligned}
& I_{L}=\frac{W_{n}-W_{p}}{W_{L}-W_{p}}>1 \\
& W_{n}>W_{L}
\end{aligned}
$$

Q. 25 A fully submerged infinite sandy slope has an inclination of $30^{\circ}$ with the horizontal. The saturated unit weight and effective angle of internal friction of sand are $18 \mathrm{kN} / \mathrm{m}^{3}$ and $38^{\circ}$, respectively. The unit weight of water is $10 \mathrm{kN} / \mathrm{m}^{3}$.Assume that the seepage is parallel to the slope. Against shear failure of the slope, the factor of safety (round off to two decimal places) is $\qquad$ .
Ans. (0.60)
Sol. $\quad \beta=30^{\circ}$

$$
\begin{aligned}
& \gamma_{s a t}=18 \mathrm{kN} / \mathrm{m}^{3} \\
& \phi=38^{0}
\end{aligned}
$$

Seepage is parallel to slope

$$
F O S=\frac{\gamma^{\prime}}{\gamma_{\text {sat }}} \frac{\tan \phi}{\tan \beta}=\frac{18-10}{18} \frac{\tan 38^{\circ}}{\tan 30^{\circ}}=0.601
$$

Q. 26 A vertical retaining wall of 5 m height has to support soil having unit weight of $18 \mathrm{kN} / \mathrm{m}^{3}$, effective cohesion of $12 \mathrm{kN} / \mathrm{m}^{2}$ and effective friction angle of $30^{\circ}$. As per Rankine's earth pressure theory and assuming that a tension crack has occurred, the lateral active thrust on the wall per meter length (in $\mathrm{kN} / \mathrm{m}$ , round off the two decimal places), is $\qquad$ .
Ans. (21.71)
Sol.


When tension crack has occurred -

$$
\begin{aligned}
P_{a} & =\frac{1}{2} K_{a} \gamma H^{2}-2 C H \sqrt{K_{a}}+\frac{2 C^{2}}{\gamma} \\
K_{a} & =\frac{1-\sin \phi}{1+\sin \phi}=\frac{1}{3} \\
\gamma & =18 \mathrm{kN} / \mathrm{m}^{3} \\
C & =12 \mathrm{kN} / \mathrm{m}^{3} \\
\phi & =30^{0} \\
P_{a} & =\frac{1}{2} \times \frac{1}{3} \times 18 \times 5^{2}-2 \times 12 \times 5 \sqrt{\frac{1}{3}}+\frac{2 \times 12^{2}}{18} \\
& =75-69.28+16 \\
& =21.72 \mathrm{kPa}
\end{aligned}
$$

Q. 27 A continuous function $f(x)$ is defined. If the third derivative at $x_{i}$ is to be computed by using the fourth order central finite-divided-difference scheme (with step length $=\mathrm{h}$ ), the correct formula is
(A) $f^{\prime \prime \prime}\left(x_{i}\right)=\frac{f\left(x_{i+3}\right)-8 f\left(x_{i+2}\right)+13 f\left(x_{i+1}\right)+13 f\left(x_{i-1}\right)-8 f\left(x_{i-2}\right)-f\left(x_{i-3}\right)}{8 h^{3}}$
(B) $f^{\prime \prime \prime}\left(x_{i}\right)=\frac{f\left(x_{i+3}\right)-8 f\left(x_{i+2}\right)-13 f\left(x_{i+1}\right)+13 f\left(x_{i-1}\right)+8 f\left(x_{i-2}\right)+f\left(x_{i-3}\right)}{8 h^{3}}$
(C) $f^{\prime \prime \prime}\left(x_{i}\right)=\frac{-f\left(x_{i+3}\right)-8 f\left(x_{i+2}\right)-13 f\left(x_{i+1}\right)+13 f\left(x_{i-1}\right)+8 f\left(x_{i-2}\right)-f\left(x_{i-3}\right)}{8 h^{3}}$
(D) $f^{\prime \prime \prime}\left(x_{i}\right)=\frac{-f\left(x_{i+3}\right)+8 f\left(x_{i+2}\right)-13 f\left(x_{i+1}\right)+13 f\left(x_{i-1}\right)-8 f\left(x_{i-2}\right)+f\left(x_{i-3}\right)}{8 h^{3}}$

Ans. (D)
Sol.

$$
\begin{aligned}
\left.\frac{\partial^{3} u}{\partial x^{3}}\right|_{x_{i}} & =\frac{-u_{i+3}+8 u_{i+2}-13 u_{i+1}+13 u_{i-1}-8 u_{i-2}+u_{i-3}}{8 \Delta h^{3}} \\
f^{\prime \prime \prime}\left(x_{i}\right) & =\frac{-f\left(x_{i+3}\right)+8 f\left(x_{i+2}\right)-13 f\left(x_{i+1}\right)+13 f\left(x_{i-1}\right)-8 f\left(x_{i-2}\right)+f\left(x_{i}-3\right)}{8 h^{3}}
\end{aligned}
$$

Q. 28 A dowel bar is placed at a contraction joint. When contraction occurs, the concrete slab cracks at predetermined location(s). Identify the arrangement, which shows the correct placement of dowel bar and the place of occurrence of the contraction crack (s).
(A)

(B)

(C)

(D)


Ans. (C)
Q. 29 A gaseous chemical has a concentration of $41.6 \mu \mathrm{~mol} / \mathrm{m}^{3}$ in air at 1 atm pressure and temperature 293 K . The universal gas constant R is $82.05 \times 10^{-6}\left(\mathrm{~m}^{3} \mathrm{~atm}\right) /(\mathrm{molK})$. Assuming that ideal gas law is valid, the concentration of the gaseous chemical (in ppm, round off to one decimal place), is $\qquad$ .
Ans. (1)
Sol.

$$
\begin{aligned}
P V & =n R T \\
V= & \frac{n R T}{P} \\
& =\frac{41.6 \times 10^{-6} \times 82.05 \times 10^{-6}}{1} \times 293 \\
& =10^{-6} \mathrm{~m}^{3}
\end{aligned}
$$

$41.6 \mu$ mole of gas volume of $10^{-6} \mathrm{~m}^{3}$

$$
\begin{aligned}
& 1 \mathrm{ppm}=\frac{1 \text { part of gas }}{10^{6} \text { parts of air }} \\
& 1 \mathrm{ppm}=\frac{41.6 \times 10^{6} \mu \text { moles }}{10^{6} \mathrm{~m}^{3}}
\end{aligned}
$$

$$
1 \mathrm{ppm}=41.6 \mu \mathrm{moles} / \mathrm{m}^{3}
$$

Q. 30 Water flows in the upward direction in a tank through 2.5 m thick sand layer as shown in the figure. The void ratio and specific gravity of sand are 0.58 and 2.7 , respectively. The sand is fully saturated. Unit weight of water is $10 \mathrm{kN} / \mathrm{m}^{3}$.


The effective stress (in $k P a$, round off to two decimal places) at point $A$, located 1 m above the base of tank, is $\qquad$ .
Ans. (8.93)
Sol. $\quad \Delta h=1.2 \mathrm{~m}, \mathrm{z}=1.5 \mathrm{~m}, L=2.5 \mathrm{~m}$

$$
\begin{aligned}
& \gamma^{\prime}=\left(\frac{G-1}{1+e}\right) \gamma_{w}=\frac{2.7-1}{1+0.58} \times 10=10.76 \mathrm{kN} / \mathrm{m}^{3} \\
& \sigma_{A}{ }^{\prime}=\gamma^{\prime} Z-i Z \gamma_{w}=10.76(1.5)-\frac{1.2}{2.5}(1.5)(10) \\
& \quad=16.14-7.2=8.94 \mathrm{kN} / \mathrm{m}^{2}
\end{aligned}
$$

Q. 31 An open traverse PQRST is surveyed using theodolite and the consecutive coordinates obtained are given in the table

| Line | Consecutive Coordinates |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
|  | Northing <br> $(\mathrm{m})$ | Southing <br> $(\mathrm{m})$ | Easting <br> $(\mathrm{m})$ | Westing <br> $(\mathrm{m})$ |
| PQ | 110.2 | - | 45.5 | - |
| QR | 80.6 | - | - | 60.1 |
| RS | - | 90.7 | - | 70.8 |
| ST | - | 105.4 | 55.5 | - |

If the independent coordinates (Northing, Easting) of station P are ( $400 \mathrm{~m}, 200 \mathrm{~m}$ ), the independent coordinates (in m ) of station T , are
(A) 405.3, 229.9
(B) $205.3,429.9$
(C) 194.7, 370.1
(D) 394.7, 170.1

Ans. (D)
Sol. $\quad \Sigma L=190.8-196.1=-5.3$
$\Sigma D=101-130.9=-29.9$
$P(400 \mathrm{~m}, 200 \mathrm{~m})$
Co-ordinate of T are $=(400-5.3,200-29.9)$

$$
=(394.7,170.1)
$$

Q. 32 The flange and web plates of the doubly symmetric built-up section are connected by continuous 10 mm thick fillet welds as shown in the figure (not drawn to the scale). The moment of inertia of the section about its principal axis $\mathrm{X}-\mathrm{X}$ is $7.73 \times 10^{6} \mathrm{~mm}^{4}$. The permissible shear stress in the fillet welds is $100 \mathrm{~N} / \mathrm{mm}^{2}$
. The design shear strength of the section is governed by the capacity of the fillet welds.


The maximum shear force (in $k N$, round off to one decimal place) that can be carried by the section, is $\qquad$ .
Ans. (393.5)
Sol. $\quad q=\frac{F A \bar{y}}{I \times b}$

$$
\begin{aligned}
& b=4 t=4 \times 0.7 \times 10=28 \\
& 100=\frac{F \times(100 \times 10) \times 55}{7.73 \times 10^{6} \times 28} \\
& F=393.527 \mathrm{kN}
\end{aligned}
$$

Q. 33 A cantilever beam PQ of uniform flexural rigidity (EI) is subjected to a concentrated moment M at R as shown in the figure


The deflection at the free end Q is
(A) $\frac{3 M L^{2}}{4 E I}$
(B) $\frac{M L^{2}}{6 E I}$
(C) $\frac{3 M L^{2}}{8 E I}$
(D) $\frac{M L^{2}}{4 E I}$

Ans. (C)
Sol.


Deflection diagram


$$
\begin{aligned}
& =\frac{M\left(\frac{l}{2}\right)^{2}}{2 E I}+\frac{M\left(\frac{l}{2}\right)}{E I}\left(\frac{l}{2}\right) \\
& =\frac{M I^{2}}{8 E I}+\frac{M I^{2}}{4 E I} \\
& =\frac{M I^{2}+2 M l^{2}}{8 E I} \\
& =\frac{3 M l^{2}}{8 E I}
\end{aligned}
$$

Q. 34 Consider the system of equations

$$
\left[\begin{array}{ccc}
1 & 3 & 2 \\
2 & 2 & -3 \\
4 & 4 & -6 \\
2 & 5 & 2
\end{array}\right]\left[\begin{array}{l}
x_{1} \\
x_{2} \\
x_{3}
\end{array}\right]=\left[\begin{array}{l}
1 \\
1 \\
2 \\
1
\end{array}\right]
$$

The value of $x_{3}$ (round off to the nearest integer), is $\qquad$ .

Ans. (3)
Sol. $[A: B]=\left[\begin{array}{ccccc}1 & 3 & 2 & : & 1 \\ 2 & 2 & -3 & : & 1 \\ 4 & 4 & -6 & : & 2 \\ 3 & 5 & 2 & : & 1\end{array}\right]$
Echelon form

$$
=\left[\begin{array}{ccccc}
1 & 3 & 2 & : & 1 \\
0 & -1 & -2 & : & -1 \\
0 & 0 & 1 & : & 0 \\
0 & 0 & 0 & : & 0
\end{array}\right]=\left[\begin{array}{ccc}
1 & 3 & 2 \\
0 & -1 & -2 \\
0 & 0 & 1 \\
0 & 0 & 0
\end{array}\right]\left[\begin{array}{c}
x_{1} \\
x_{2} \\
x_{3}
\end{array}\right]=\left[\begin{array}{c}
1 \\
-1 \\
3 \\
0
\end{array}\right]
$$

$\Rightarrow \quad x_{3}=3$
Q. 35 A simply supported prismatic concrete beam of rectangular cross- section, having a span of 8 m , is prestressed with an effective prestressing force of 600 kN . The eccentricity of the prestressing tendon is zero at supports and varies linearly to a value of e at the mid-span. In order to balance an external concentrated load of 12 kN applied at the mid-span, the required value of e (in mm, round off to the nearest integer) of the tendon, is $\qquad$ .
Ans. (40)
Sol.


$$
\begin{aligned}
& P \cos \theta \times e=\frac{W l}{4} \\
& 60 \times 1 \times 2=\frac{12 \times 8}{4} \\
& e=\frac{24}{600} \times 100 \\
& e=40 \mathrm{~mm}
\end{aligned}
$$

Q. 36 The total stress paths corresponding to different loading conditions, for a soil specimen under the isotropically consolidated stress state ( O ), are shown below


| Stress Path | Loading Condition |
| :---: | :---: |
| OP | I-Compression loading ( $\sigma_{1}-$ increasing; $\sigma_{3}-$ constant $)$ |
| OQ | II-Compression unloading ( $\sigma_{1}-$ constant; $\sigma_{3}-$ decreasing $)$ |
| OR | III-Extension unloading ( $\sigma_{1}-$ decreasing; $\sigma_{3}$ - constant) |
| OS | IV-Extension loading ( $\sigma_{1}-$ constant; $\sigma_{3}$ - increasing) |

The correct match between the stress paths and the listed loading conditions, is
(A) OP-III, OQ-II, OR-I, OS-IV
(B) OP-I, OQ-II, OR-IV, OS-III
(C) OP-IV, OQ-III, OR-I, OS-II
(D) OP-I, OQ-III, OR-II, OS-IV

Ans. (C)
Sol. I. Compression loading

II. Compression unloading


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III. Extension unloading

IV. Extension loading

Q. 37 Distributed load (s) of $50 \mathrm{kN} / \mathrm{m}$ may occupy any position (s) (either continuously or in patches) on the girder PQRST as shown in the figure (not drawn to the scale)


The maximum negative (hogging) bending moment (in $\mathrm{kN} . \mathrm{m}$ ) that occurs at point R , is
(A) 150.00
(B) 56.25
(C) 22.50
(D) 93.75

Ans. (B)
Sol. ILD for $M_{s}$
 $50 \mathrm{kN} / \mathrm{m}$

$\therefore M_{\max }$ at support $=-\left[\frac{1}{2} \times 1.5 \times 1.5 \times 50\right]$
$=-56.25 \mathrm{kN}-\mathrm{m}$ or $56.25 \mathrm{kN}-\mathrm{m}$ (hogging)
ILD for BM at the centre of span 'QS'

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steps to success...

$M_{c}=-\left[\frac{1}{2} \times 1.5 \times 0.75 \times 50\right] \times 2$
$=-56.25 \mathrm{kN}-\mathrm{m}$ or $56.25 \mathrm{kN}-\mathrm{m}$ (hogging)


So, it can be concluded that for maximum hogging moment, overhang span ( $P Q \& S T$ ) can be loaded and because of symmetry, magnitude of maximum hogging moment remains same throughout span QS.
Q. 38 Water flows at the rate of $12 \mathrm{~m}^{3} / \mathrm{s}$ in a 6 m wide rectangular channel. A hydraulic jump is formed in the channel at a point where the upstream depth is 30 cm (just before the jump). Considering acceleration due to gravity as $9.81 \mathrm{~m} / \mathrm{s}^{2}$ and density of water as $1000 \mathrm{~kg} / \mathrm{m}^{3}$, the energy loss in the jump is
(A) 114.2 MW
(B) $141.2 \mathrm{~J} / \mathrm{s}$
(C) 114.2 kW
(D) $141.2 \mathrm{~h} . \mathrm{p}$

Ans. (C)
Sol. Assuming channel bed to be horizontal and frictionless,

$$
q=\frac{12}{6}=2 \mathrm{~m}^{3} / \mathrm{s} / \mathrm{m}
$$



Initial Froude No. $\left(F_{r}\right)=\left(\frac{q^{2}}{g Y_{1}^{3}}\right)^{1 / 2}$

$$
=\left(\frac{2^{2}}{9.81 \times 0.3^{3}}\right)^{1 / 2}=3.88
$$

From Ballenger's Momentum equation for a rectangular channel

$$
\begin{aligned}
\frac{Y_{2}}{Y_{1}}= & \frac{1}{2}\left(-1+\sqrt{1+8 F_{1}^{2}}\right) \\
& =\frac{1}{2}\left(-1+\sqrt{1+8 \times 3.88^{2}}\right)=5.018
\end{aligned}
$$

$$
Y_{2}=5.018 \times 0.3=1.505 \mathrm{~m}
$$

Head loss in the jump $\left(h_{L}\right)=\frac{\left(Y_{2}-Y_{1}\right)^{3}}{4 Y_{1} Y_{2}}$

$$
\begin{aligned}
& =\frac{(1.505-0.3)^{3}}{4 \times 1.505 \times 0.3} \\
& =0.968 \mathrm{~m}
\end{aligned}
$$

Power lost in the jump $=\gamma_{w} Q h_{L}$

$$
\begin{aligned}
& =(9.81 \times 12 \times 0.968) \mathrm{kW} \\
& =114.04 \mathrm{~kW}
\end{aligned}
$$

Q. 39 The relationship between traffic flow rate (q) and density (D) is shown in the figure


The shock wave condition is depicted by
(A) Flow changing from point 3 to point $7\left(q_{3}<q_{7}\right)$
(B) Flow with respect to point 4 and point $5\left(q_{4}=q_{5}\right)$
(C) Flow changing from point 2 to point $6\left(q_{2}>q_{6}\right)$
(D) Flow with respect to point $1\left(q_{1}=q_{\max }\right)$

Ans. (*)
Sol.

Q. 40 A stream with a flow rate of $5 \mathrm{~m}^{3} / \mathrm{s}$ is having an ultimate BOD of $30 \mathrm{mg} /$ litre. A wastewater discharge of $0.20 \mathrm{~m}^{3} / \mathrm{s}$ having $\mathrm{BOD}_{5}$ of $500 \mathrm{mg} /$ litre joins the stream at a location and instantaneously gets mixed up completely. The cross-sectional area of the stream is $40 \mathrm{~m}^{2}$ which remains constant. BOD exertion rate constant is 0.3 per day (logarithm base to e). The BOD (in $\mathrm{mg} / \mathrm{litre}$, round off to two decimal place) remaining at 3 km downstream from the mixing location, is $\qquad$ .
Ans. (49.57)
Sol. $\quad t=\frac{d}{v}$
Where, $v=\frac{Q_{S}+Q_{R}}{A}=\frac{0.2+5}{40}=0.13 \mathrm{~m} / \mathrm{sec}$

$$
\begin{aligned}
& t=\frac{3 \times 10^{3}}{0.13 \times 86400}=0.26 \text { days } \\
& B O D_{5}=B O D_{u}\left(1-e^{-k \times 5}\right) \\
& B O D_{u}=\frac{500}{\left(1-e^{-0.3 \times 5}\right)}=643.66 \mathrm{mg} / 1 \\
& D O_{\text {mix }}=\frac{Q_{R} B O D_{u}+Q_{S} B O D_{u}}{Q_{S}+Q_{R}} \\
& \quad=\frac{5 \times 30+0.2 \times 643.66}{5+0.2}=53.6 \mathrm{mg} / 1 \\
& \begin{array}{r}
L_{t}= \\
L_{0} e^{-k \times t} \\
\quad=53.6 e^{-0.3 \times 0.26}=49.57 \mathrm{mg} / \mathrm{l}
\end{array}
\end{aligned}
$$

Q. 41 Three reservoirs $\mathrm{P}, \mathrm{Q}$ and R are interconnected by pipes as shown in the figure (not drawn to the scale). Piezometric head at the junction $S$ of the pipes is 100 m . Assume acceleration due gravity as $9.81 \mathrm{~m} / \mathrm{s}^{2}$ and density of water as $1000 \mathrm{~kg} / \mathrm{m}^{3}$. The length of the pipe from junction $S$ to the inlet of reservoir $R$ is 180 m .


Considering head loss only due to friction (with friction factor of 0.03 for all the pipes), the height of water level in the lowermost reservoir R (in $m$, round off to one decimal place) with respect to the datum is $\qquad$ .
Ans. (97.5)
Sol. Apply continuity

$$
\begin{aligned}
Q_{3}= & Q_{1}+Q_{2} \\
& =A_{1} V_{1}+A_{2} V_{2} \\
& =\frac{\pi}{4}(0.3)^{2}(2.56)+\frac{\pi}{4}(0.3)^{2}(1.98) \\
& =0.3209 \mathrm{~m}^{3} / \mathrm{s}
\end{aligned}
$$

Apply energy equation between (S) and (R)

$$
\begin{aligned}
& H_{s}=H_{r}+h_{f} \\
& 100=z+\frac{8 Q_{3}^{2}}{\pi^{2} g} \times \frac{f L_{3}}{D_{3}^{5}} \\
& 100=z+\frac{8(0.3209)^{2}}{\pi^{2} g} \times \frac{(0.03)(180)}{(0.45)^{5}} \\
& z=97.51 \mathrm{~m}
\end{aligned}
$$

Q. 42 A water supply scheme transports 10 MLD (Million Litres per Day) water through a 450 mm diameter pipeline for a distance of 2.5 km . A chlorine dose of $3.50 \mathrm{mg} /$ litre is applied at the starting point of the pipeline to attain a certain level of disinfection at the downstream end. It is decided to increase the flow rate from 10 MLD to 13 MLD in the pipeline. Assume exponent for concentration, $n=0.86$. With this increased flow, in order to attain the same level of disinfection, the chlorine dose (in $\mathrm{mg} / \mathrm{litre}$ ) to be applied at the starting point should be
(A) 4.75
(B) 3.95
(C) 5.55
(D) 4.40

Ans. (A)
Sol. We know,

$$
\begin{aligned}
& t c^{n}=k \\
& n=0.86 \\
& t_{1}\left(c_{1}\right)^{n}=t_{2}\left(c_{2}\right)^{n} \\
& \frac{A L}{Q_{1}}\left(c_{1}\right)^{n}=\frac{A L}{Q_{2}}\left(c_{2}\right)^{n} \\
& \frac{1}{10}(3.5)^{0.86}=\frac{1}{13}\left(c_{2}\right)^{0.86} \\
& 0.29636=\frac{1}{13}\left(c_{2}\right)^{0.86} \\
& C_{2}=4.748 \simeq 4.75 \mathrm{mg} / \mathrm{L}
\end{aligned}
$$

Q.43 In a homogeneous unconfined aquifer of area $3.00 \mathrm{~km}^{2}$, the water table was at an elevation of 102.00 m . After a natural recharge of volume 0.90 million cubic meter $\left(\mathrm{Mm}^{3}\right)$, the water table rose to 103.20 m After this recharge, ground water pumping took place and the water table dropped down to 101.20 m . The volume of ground water pumped after the natural recharge, expressed (in $\mathrm{Mm}^{3}$ and round off to two decimal places), is $\qquad$ .
Ans. (1.5)
Sol.
$\qquad$


$$
\begin{aligned}
& V_{R}=0.9 \mathrm{Mm}^{3} \\
& \begin{array}{l}
V=3 \times(103.2-102) \\
\quad=3 \times 1.2=3.6 \mathrm{Mm}^{3} \\
y_{s} \text { or } y_{R}=\frac{V R}{V}=\frac{0.9}{3.6}
\end{array}
\end{aligned}
$$

Now, $y_{s}=\frac{V_{D}}{V}$

$$
\begin{aligned}
& V_{D}=\frac{0.9}{3.6}[3 \times(103.2-101.2)] \\
& V_{D}=1.5 \mathrm{Mm}^{3}
\end{aligned}
$$

Q. 44 The lengths and bearings of a traverse PQRS are:

| Segment | Length (m) | Bearing |
| :---: | :---: | :---: |
| PQ | 40 | $80^{0}$ |
| QR | 50 | $10^{\circ}$ |
| RS | 30 | $210^{\circ}$ |

The length of line segment SP (in m, round off to two decimal places), is $\qquad$ .
Ans. (44.79)
Sol.

| Segment | Length | Bearing | Lat. $(l \cos \theta)$ | Dep. $(l \sin \theta)$ |
| :---: | :---: | :---: | :---: | :---: |
| PQ | 40 m | $80^{\circ}$ | 6.945 | 39.392 |
| QR | 50 m | $10^{\circ}$ | 49.250 | 8.682 |
| RS | 30 m | $210^{\circ}$ | -25.980 | -15 |
| SP | 1 | $\theta$ | $l \cos \theta$ | $l \sin \theta$ |

$$
\begin{aligned}
& \Sigma L=l \cos \theta+30.215=0 \\
& \Sigma D=l \sin \theta+33.081=0 \\
& l \cos \theta=-30.215 \\
& l \sin \theta=-33.081 \\
& l=\sqrt{(30.215)^{2}+(33.081)^{2}}=44.802 \mathrm{~m}
\end{aligned}
$$

Q. 45 Surface Overflow Rate (SOR) of a primary settling tank (discrete settling) is $20000 \mathrm{litre} / \mathrm{m}^{2}$ per day. Kinematic viscosity of water in the tank is $1.01 \times 10^{-2} \mathrm{~cm}^{2} / \mathrm{s}$. Specific gravity of the settling particles is 2.64. Acceleration due to gravity is $9.81 \mathrm{~m} / \mathrm{s}^{2}$. The minimum diameter (in $\mu \mathrm{m}$, round off to one decimal place) of the particles that will be removed with $80 \%$ efficiency in the tank, is $\qquad$ .
Ans. (14.53)
Sol. $\quad V=20 \mathrm{~m} /$ day,$\eta=80 \%=\frac{V}{V} \times 100$

$$
\begin{aligned}
& V=\frac{0.8 \times 2000 \times 10^{-3}}{86400}=1.85 \times 10^{-4} \mathrm{~m} / \mathrm{sec}=\frac{(G-1) \gamma_{w} d^{2}}{18 \mu} \\
& V=\frac{\mu}{P} \\
& \mu=1.02 \times 10^{-2} \times 10^{-4} \times 10^{3}=1.02 \times 10^{-3} \\
& 1.85 \times 10^{-4}=\frac{(2.64-1) \times\left(9.81 \times 10^{3}\right) \times d^{2}}{18 \times 1.02 \times 10^{-3}} \\
& d=1.453 \times 10^{-5} \mathrm{~m}=14.53 \mu \mathrm{~m}
\end{aligned}
$$

Q. 46 A rigid weightless platform PQRS shown in the figure (not drawn to the scale) can slide freely in the vertical direction. The platform is held in position by the weightless member $\mathbf{O J}$ and four weightless, frictionless rollers, Points $\mathbf{O}$ and $\mathbf{J}$ are pin connections. A block of 90 kN rests on the platform as shown in the figure.


The magnitude of horizontal component of the reaction (in kN ) at pin $\mathbf{O}$, is
(A) 150
(B) 120
(C) 90
(D) 180

Ans. (B)
Sol.


$$
\begin{aligned}
& \Sigma y=0 \\
& R_{0} \sin 36.87-90=0 \\
& R_{0}=\frac{90}{\sin 3687^{0}}=150 \mathrm{kN}
\end{aligned}
$$

Horizontal reaction at $O=H_{0}$

$$
\begin{aligned}
& =R_{0} \cos 36.87=150 \times \cos 36.87 \\
& =120 \mathrm{kN}
\end{aligned}
$$

Q. 47 The singly reinforced concrete beam section shown in the figure (not drawn to the scale) in made of M25 grade concrete and Fe500 grade reinforcing steel. The total cross-sectional area of the tension steel is $942 \mathrm{~mm}^{2}$.


As per Limit State Design of IS 456:2000, the design moment capacity (in kN.m, round off to two decimal places) of the beam section, is $\qquad$ .
Ans. (158.27)
Sol. Given section


$$
\begin{aligned}
& A_{s t}=942 \mathrm{~mm}^{2} \\
& f_{y}=500 \mathrm{~N} / \mathrm{mm}^{2} \\
& f_{c k}=25 \mathrm{~N} / \mathrm{mm}^{2}
\end{aligned}
$$

Limiting depth of N.A -

$$
\begin{aligned}
& \begin{array}{l}
x_{u \text { lim }}=0.46 \mathrm{~d} \\
=0.46 \times 450=207 \mathrm{~mm} \\
x_{u}=
\end{array} \quad \quad \text { (For Fe } 41 \\
& 0.87 f_{y} A_{s t} \\
& 0.36 f_{c k}
\end{aligned} \frac{0.87 \times 500 \times 942}{0.36 \times 25 \times 300}=151.76 \mathrm{~mm}
$$

Section is under - reinforced section

$$
\begin{aligned}
& x_{u}<x_{u \lim } \\
& M_{u}=0.36 f_{c k} b x_{u}\left(d-0.42 x_{u}\right) \\
& M_{u}=0.36 \times 25 \times 300 \times 151.76(450-0.42 \times 151.67) \\
& \quad M_{u}=158.271 \mathrm{kNm}
\end{aligned}
$$

Q. 48 If C represents a line segment between $(0,0,0)$ and $(1,1,1)$ in Cartesian coordinate system, the value (expressed as integer) of the line integral

$$
\int_{c}[(y+z) d x+(x+z) d y+(x+y) d z] \text { is }
$$

$\qquad$ .
Ans. (3)
Sol.

$$
\begin{aligned}
& I=\int_{c}[(y d x+x d y)+(z d x+x d z)+(z d y+y d z)] \\
& =\int_{c}[d(x y)+d(x z)+d(y z)] \\
& =(x y+y z+z x)_{(0,0,0)}^{(1,1,1)} \\
& =(1+1+1)-(0+0+0) \\
& =3
\end{aligned}
$$

Q. 49 A circular water tank of 2 m diameter has a circular orifice of diameter 0.1 m at the bottom. Water enters the tank steadily at a flow rate of 20 litre/s and escapes through the orifice. The coefficient of discharge of the orifice is 0.8 . Consider the acceleration due to gravity as $9.81 \mathrm{~m} / \mathrm{s}^{2}$ and neglect frictional losses. The height of the water level (in $m$, round off to two decimal places) in the tank at the steady state, is
$\qquad$ .

Ans. (0.52)
Sol. Discharge through orifice $=$ Water enters in the tank

$$
\begin{aligned}
& c_{d} a \sqrt{2 g H}=20 \times 10^{-3} \\
& 0.8 \times \frac{\pi}{4}(0.1)^{2} \sqrt{2 g H}=0.02 \\
& H=0.5164 \mathrm{~m}
\end{aligned}
$$

Q. 50 A 10 m thick clay layer is resting over a 3 m thick sand layer and is submerged. A fill of 2 m thick sand with unit weight of $20 \mathrm{kN} / \mathrm{m}^{3}$ is placed above the clay layer to accelerate the rate of consolidation of the clay layer. Coefficient of consolidation of clay is $9 \times 10^{-2} \mathrm{~m}^{2} /$ year and coefficient of volume compressibility of clay is $2.2 \times 10^{-4} \mathrm{~m}^{2} / \mathrm{kN}$. Assume Taylor's relation between time factor and average degrees of consolidation.


The settlement(in mm, round off to two decimal place) of the clay layer, 10 years after the construction of the fill, is $\qquad$ -
Ans. (18.83)
Sol. $\Delta \bar{\sigma}=2 \times 20=40 \mathrm{kN} / \mathrm{m}^{2}$

$$
\begin{aligned}
\Delta H & =m_{v} \Delta \bar{\sigma} H \\
& =2.2 \times 10^{-4} \times 40 \times 10 \times 10^{3} \mathrm{~mm} \\
T_{v}= & \frac{C \times t}{H^{2}}=\frac{9 \times 10^{-2} \times 10}{5^{2}}=0.036
\end{aligned}
$$

$$
T_{v}=\frac{\pi}{4} U^{2}
$$

$$
U=\sqrt{\frac{0.036 \times 4}{\pi}}=0.214
$$

$\Delta h$ after 10 years $=0.214 \times 88=18.832 \mathrm{~mm}$
Q. 51 A rigid, uniform, weightless, horizontal bar is connected to three vertical members $P, Q$ and $R$ as shown in the figure (not drawn to the scale). All three members have identical axial stiffness of $10 \mathrm{kN} / \mathrm{mm}$. The lower end of bars P and R rest on a rigid horizontal surface. When NO load is applied, a gap of 2 mm
exists between the lower end of the bar Q and the rigid horizontal surface. When a vertical load W is placed on the horizontal bar in the downward direction, the bar still remains horizontal and gets displaced by 5 mm in the vertically downward direction.


The magnitude of the load W (in $k N$, round off to the nearest integer), is $\qquad$ .
Ans. (130)
Sol.

Q. 52 The soil profile at a site up to a depth of 10 m is shown in the figure (not drawn to the scale). The soil is preloaded with a uniform surcharge $(q)$ of $70 \mathrm{kN} / \mathrm{m}^{2}$ at the ground level. The water table is at a depth of 3 m below ground level. The soil unit weight of the respective layers is shown in the figure. Consider unit weight of water as $9.81 \mathrm{kN} / \mathrm{m}^{3}$ and assume that the surcharge $(q)$ is applied instantaneously.

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Immediately after preloading, the effective stresses (in kPa ) at points $P$ and $Q$ respectively, are
(A) 124 and 204
(B) 54 and 95
(C) 36 and 126
(D) 36 and 90

Ans. (B)
Sol.


At point $\mathrm{P}, q=70 \mathrm{kN} / \mathrm{m}^{2}$

$$
\begin{aligned}
& r_{w}=9.81 \mathrm{kN} / \mathrm{m}^{3} \\
& (3 \times 18)+(70-70)=54
\end{aligned}
$$

At point $\mathrm{Q},(3 \times 18)+(70)+(4 \times 20)-[(70)+(9.81) \times 4]$
204-109.24
$94.76 \approx 95$
Q. 53 The appropriate design length of a clearway is calculated on the basis of 'Normal Take-off' condition. Which one of the following options correctly depicts the length of the clearway? (Note : None of the options are drawn to scale)


## Ans. (B)

Sol. For normal take off case :
Clearway $<\frac{1}{2}$ (1.5 takeoff distance -1.15 of lift off distance $)$

$$
\begin{aligned}
& <\frac{1}{2}(1.15 \times 1625-1.15 \times 875) \\
& <431.25 \mathrm{~m}
\end{aligned}
$$

Q. 54 For the ordinary differential equation $\frac{d^{2} x}{d t^{2}}-5 \frac{d x}{d t}+6 x=0$, with initial conditions $x(0)=0$ and $\frac{d x}{d t}(0)=10$ , the solution is
(A) $10 e^{2 t}+10 e^{3 t}$
(B) $5 e^{2 t}+6 e^{3 t}$
(C) $-10 e^{2 t}+10 e^{3 t}$
(D) $-5 e^{2 t}+6 e^{3 t}$

Ans. (C)

Sol. Auxilary equation is $m^{2}-5 m+6=0$

$$
m=2,3
$$

So, C.F. $=c_{1} e^{2 t}+c_{2} e^{3 t}$
$\mathrm{PI}=0$
Solution is $x=C E+P I$

$$
=C_{1} e^{2 t}+C_{2} e^{3 t}
$$

And $\frac{d x}{d t}=2 C_{1} e^{2 t}+3 C_{2} e^{3 t}$

$$
\begin{aligned}
& C_{1}=-10, C_{2}=10 \\
& x=-10 e^{2 t}+10 e^{3 t}
\end{aligned}
$$

Q. 55 Traffic volume count has been collected on a 2-lane road section which needs upgradation due to severe traffic flow condition. Maximum service flow rate per lane is observed as $1280 \mathrm{veh} / \mathrm{h}$ at level of service 'C'. The Peak Hour Factor is reported as 0.78125 . Historical traffic volume count provides Annual Average Daily Traffic as 12270 veh/day, Directional split of the traffic flow is observed to be 60:40. Assuming that traffic stream consists of 'All Cars' and all drivers are 'Regular Commuters', the number of extra lane(s) (round off to the next higher integer) to be provided, is $\qquad$ .

Ans. (6)
Sol. Number of the needed given LOS

$$
=\frac{D D H V}{P H F \times M S F \times F_{H V} \times f_{p}}
$$

Where, $D D H V=$ Directional distribution hour volume
$P H F=$ Peak hour factor
MSF = Maximum service flow rate of Los
$F_{H V}=$ Heavy vehicle familiarity adjustment factor
$f_{p}=$ Road under familiarity adjustment factor
$D D H V=122270 \times 0.6=7362 \mathrm{Vah} /$ day $(0.6-$ max. directional distributed $)$
$F_{H V}=f_{p}=1$ as per given condition

$$
N=\frac{7362}{0.78125 \times 1280 \times 1 \times 1}=7.362=8 \text { lanes }
$$

Number of extra lanes $=8-2=6$ lanes

